Reducing Concrete Damage through Corrosion Resistant Reinforcing: an Overview of 40 years of Research and Practice

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Abstract

• Over the past 40 years, substantial research has been conducted into reinforcing materials and coatings to reduce corrosion damage of concrete structures. This paper will present information on research and modifications to fusion-bonded epoxy-coated steel during this period and compare its performance in recently completed tests and extensive data analysis with that of other products.

Epoxy Bar Use

- 2nd most common strategy to prevent reinforcement corrosion
 - Following increased concrete cover
- 850,000,000 ft² of decks
 - >70,000 bridges in the US alone
 - ~600,000 ton/yr or 10 15% of all rebar in NA
- USA, Canada, Middle East, Japan, and India



1970's use of deicing salts

- "The premature deterioration of concrete bridge deckings, in 5-10 years, has become a major problem during the past decade.
- ...caused by chloride ions from deicing materials.
- Use of deicing materials,...has increased substantially during the past decade."



1974 National Bureau of Standards

- Clifton, Beeghly, Mathey
- 47 coating materials (15 epoxies)
- Epoxy coating should adequately protect steel reinforcing bars from corrosion with acceptable bond and creep characteristics



First Bridge 1973 Pennsylvania

1983 FHWA-non-specification epoxycoated bars

Horrible bars

- 3-year-old bars
- >25 holidays/ft
- up to 0.8% damage
- failed bent test
- coating easy to peel
- bars stored outdoors 2 year
- 15lb/cy chloride in concrete
- 1"cover, w/c 0.53
- Deemed "very effective",
 - Significantly reduced corrosion, esp. when used double mats.
 - 12 to 46 times less current than black bars

"if its green its good"



1988 KCC Research

- 3-year evaluation of bent and straight bars from 7 suppliers
 - Performed well during 1.35 years of southern exposure
 - During a subsequent 10.5 months water ponding, bars from two sources remained passive while many bars from other sources started to corrode
 - The technology of epoxy-coated rebars, as practiced in North America, is flawed

1991 WJE Evaluations

• Some provide high electrical resistance

 Some coatings are inferior and provide low electrical resistance



Epoxy performance depends on manufacture

1991 CRSI Voluntary Certification Program for manufacturing





1998 New Breeds of Reinforcing Bars

- McDonald, Sherman, Pfeifer
- Screening test: 33 organic coated, 14 ceramic, inorganic and metallic coated, 10 solid types
- Concrete test: 10 type of bars
 - Epoxy coated bars
 - Galvanized
 - Zinc-alloy clad
 - Copper clad
 - Type 304 SS
 - Type 316 SS









UNIVERSITY OF KANSAS STUDIES

2010 – University of Kansas



2010 – University of Kansas

Cracked and uncracked concrete

E.

- Long term testing
- Damaged bars





Corrosion-Initiation Beam (CI) Specimen



Measured Corrosion Thresholds

System	Threshold (lb/yd³)	Relative threshold
Uncoated	1.58	1
Epoxy Coated	7.28	4.6
Туре 2205	26.4	16.7

Chloride Data at cracks 3 in. depth, AADT > 7500



C(t) = 0.0316.t + 0.746

Where t = time (months) C(t) = chloride content (lb/yd³)

Estimated performance – cracked concrete

	Initiation (years)
Uncoated reinforcing	2
Epoxy-coated reinforcing	20
Type 2205 stainless-steel	68

Estimated performance – cracked concrete

	Initiation (years)	Propagation (years)
Uncoated reinforcing	2	7
Epoxy-coated reinforcing	20	25
Type 2205 stainless-steel	68	359

Estimated performance cracked concrete

	Initiation (years)	Propagation (years)	Time to first repair (years)
Uncoated reinforcing	2	7	14
Epoxy-coated reinforcing	20	25	50
Type 2205 stainless-steel	68	359	432

Time to repair = initiation + propagation + 5 years

Initial Cost



Life-cycle cost



FHWA TURNER FAIRBANKS

2012 – FHWA/Lee study

- 12 different bar types from 11 sources
 - Epoxy-coated*
 - Dual-clad*
 - Galvanized*

- Low carbon chromium
- Steel alloys
- Stainless clad
- 2205 Stainless steel



Preliminary Findings

- Use of fusion-bonded coated bars (epoxy, and dual coated) in both mats offered the best corrosion resistance
 - Due to large electrical resistance
- Alloyed bars did not provide adequate corrosion resistance
 - A1035 low carbon-chrome
 - Duracorr
 - 3CR12

Preliminary Findings (con't)

 Solid stainless and stainless clad bars exhibited very good corrosion performance

– Enduramet 32?

 Galvanized bars may be used in moderately corrosive environments

Final report due 2013?

FIELD STUDIES



- Four bridges
 - 1973 to 1978
- Overall condition
 - good to very good, with no or modest levels of corrosion activity.
- Corrosion constrained joints over piers
- Amount of delamination in all decks is very low



Chloride threshold greater than black

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New York State Department of Transportation 2009

- Statistical analysis of 17,000 structures
- Structural decks with epoxy-coated reinforcement perform significantly better than those with uncoated reinforcement, especially in the later years.



West Virginia Bridges



- WVDOT survey in 1993
 - Built mid 1970's
 - 33 decks
- 2009 Studies
 - All decks with black bars had been overlaid



Side-by-side analysis of black vs epoxy

West Virginia 2009 – 34 yo deck









2010 Boatman/Michigan DOT

- The service life of a black rebar bridge deck is estimated to be 35 years.
- The service life of an ECR bridge deck is estimated to be approximately 70 years.



2012 Hatami and Morcous

- Rating of 5 as minimum acceptable
 - Black bar 40 years
 - Epoxy bar 68 years
- Rating of 4 as minimum acceptable
 - Black bar 60 years
 - Epoxy Bar 90 years



1986 Florida Bridges



- Concrete spalling within 6 years of placement
- Cover = 1 in.
- Extremely high chloride (23 lb/cy)

2011 Florida Predictions (Sagues et al)



Most bridges in Florida with epoxy-coated bars are predicted to last 100 years with minimal corrosion damage

CURRENT KNOWLEDGE

Epoxy-coated reinforcing steel

- A775: Green
 - Bent after coating
 - Most widely used and researched material
 - Significant material improvements over 37 years
 - Over 70,000 bridges
 - ~ 2500 per year
- A934: Purple or Grey
 - Bent before coating





How do you do to ensure your paint is durable?

- Preparation
- Material
- Application

Quality changes since 1990

- Increased coating thickness
- Required roughness
- Clean substrate (backside contamination)
- No cracking allowed during bending
- Improved storage and handling



The 2012 product is greatly improved

Coating thickness

1980s

• 7 +/- 2 mil

2010

- 7 12 mil
 - For #3, #4 and #5
- 7 16 mil
 - For #6 and greater



Backside contamination

 1992: Median contamination was 25%...from 10 to 70%

2011: Average contamination less than 15%





Anchor profile - roughness



Profile not measured in 1980s

Bending

- 1992: ...zero to 32 cracks at the bends

 Bending to 120°
- 2011: Cracks in coating not allowed
 - Bending to 180°





Design

- Increase lap length
 Follow code
 recommendations
- Use in both mats to provide long life



FIELD OPERATIONS

Optimum performance

- Like all materials
- Appropriate handling
 Damaged bars have lower

performance than

undamaged bars



Damaged bars still perform well

Field specifications

- Individual agency specifications
- ACI 301 Specifications for Structural Concrete
- ASTM A775 (AASHTO M284) Annex
- ASTM A934 Annex
- ASTM D3963 (AASHTO M317)

Pick up

- Spreader bar or strongback with multiple pickup points
 - minimize sag



Carrying

DO NOT DRAG!



Slings

- Use nylon or padded slings
- No bare chains or cables



Storage

Store bundles on timber cribbing
 – Do not store directly on the ground



Placement

- Oil forms prior to bar placement
- Use coated supports
 - epoxy or plastic
- Tie using coated tie wire



Cutting

- Cut using power shears or chop saws
- Do not flame cut



Jobsite bending

- Only bend with the permission of the engineer
 - Bend at ambient temperatures
 - Use CRSI Bending diameters



Rejection

• > 2% area damaged in any 1 ft section



- < 5% of patching material in any 1 ft section
 - Does not include sheared or cut ends

Coating repair

- Wire brush
 - Remove rust or other contaminants
- Two-part epoxy
 - No spray cans
 - Approved by the coating manufacturer
 - Mixed according to the manufacturer's direction
 - Used within the specified pot life
- Provided cure time prior to concrete







Pre-Pour inspection

- Bar spacing, size and type
- Lap lengths
- Bar supports
- Mechanical splices
- Coated tie wire



Pre-Pour inspection

- Repairs
 - Sufficient time for coatings to dry
 - Use a two-part epoxy, not spray can
- Clear concrete cover
 - If force is used do not damage bars



Traffic and welds

- Minimize traffic over the epoxycoated steel
- Weld only with the permission of the engineer
 - Cleaned and patched with repair materials
- Stands or rails used for concrete placement machines should not be welded to the epoxy-coated steel



Concrete hose

- Fit concrete pumps with an "S" bend
- Protect bar against hose couplers



Vibration

• Use plastic headed vibrators





Additional information

Inspectors



Field Crews







Woodrow Wilson Bridge, Virginia/Maryland



I-35 Minneapolis, Minnesota





Bridge of Honor, Ohio





Biloxi Bay Bridge, Mississippi







World Trade Center, NY

Georgia Aquarium



Aqua Building Chicago



Crystal Bridges Museum of American Art - Bentonville, Arkansas

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